# ON BIG DEFLECTIONS OF CUSPED PLATES 

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Abstract: In the first part of the present paper we derive general equations for big deflections of cusped plates (i.e., symmetric prismatic shells), following an approach used for the case of plates of the constant thickness (see Timoshenko, Woinowsky-Krieger [1], §101). These equations represent a nonlinear system. The second part is devoted to the cylindrical bending of such plates.

Key words: cusped plates, cusped prismatic shells, big deflections, mathematical modelling.

MSC 2000: 74K20, 74K25, 74B20

## 1.The model of cusped plates with big deflections

In case of symmetric cusped shells (see Vekua [2]) the elastic body under consideration is bounded from the top and the bottom by the surfaces $z=h(x, y)$ and $z=-h(x, y)$, respectively, and from the lateral side by the cylindrical surface parallel to the axis $O z$. The shell thickness $2 h(x, y) \geq 0$ and case $2 h(x, y)=0$ can occur only on the projection boundary of the shell. The last part of the boundary is called a cusped edge of the shell. Let us consider bending of a cusped shell caused besides of transverse loadings by the forces acting in the middle plane of the cusped shell. Further, let us consider equilibrium of a small element cut out of the shell by two pairs of planes parallel to the coordinate planes $O x z$ and $O y z$. On the Figures 1-3 are shown, acting on the above element (see Donnell [3], p.220), bending ( $M_{x}, M_{y}$ ) and twisting $\left(M_{x y}, M_{y x}\right)$ moments, shearing $\left(Q_{x}, Q_{y}\right)$ forces, and forces ( $\left.N_{x}, N_{y}, N_{x y}=N_{y x}\right)$, acting in the middle plane. Projecting all the forces to the axes $O z, O x, O y$, we get, correspondingly,

$$
\begin{aligned}
\frac{\partial Q_{x}}{\partial x} d x d y+\frac{\partial Q_{y}}{\partial y} d y d x+q d x d y & =0 \\
\frac{\partial N_{x}}{\partial x} d x d y+\frac{\partial N_{x y}}{\partial y} d y d x+q_{x} d x d y & =0 \\
\frac{\partial N_{x y}}{\partial x} d x d y+\frac{\partial N_{y}}{\partial y} d y d x+q_{y} d x d y & =0
\end{aligned}
$$

[^0]where $\left(q_{x}, q_{y}, q\right)$ is the intensity of loading ${ }^{2)}$. Hence,
\[

$$
\begin{gather*}
\frac{\partial Q_{x}}{\partial x}+\frac{\partial Q_{y}}{\partial y}+q=0  \tag{1.1}\\
\frac{\partial N_{x}}{\partial x}+\frac{\partial N_{x y}}{\partial y}+q_{x}=0  \tag{1.2}\\
\frac{\partial N_{x y}}{\partial x}+\frac{\partial N_{y}}{\partial y}+q_{y}=0 \tag{1.3}
\end{gather*}
$$
\]

Taking moments with respect to the axes $O x$ and $O y$ of all the forces, acting on the element under consideration, and neglecting quantities of the third order smallness, we obtain, correspondingly

$$
\begin{aligned}
& \frac{\partial M_{x y}}{\partial x} d x d y-\frac{\partial M_{y}}{\partial y} d y d x+Q_{y} d x d y=0 \\
& \frac{\partial M_{y x}}{\partial y} d y d x+\frac{\partial M_{x}}{\partial x} d x d y-Q_{x} d x d y=0
\end{aligned}
$$

Therefore,

$$
\begin{align*}
& \frac{\partial M_{x y}}{\partial x}-\frac{\partial M_{y}}{\partial y}+Q_{y}=0  \tag{1.4}\\
& \frac{\partial M_{x}}{\partial x}+\frac{\partial M_{y x}}{\partial y}-Q_{x}=0 \tag{1.5}
\end{align*}
$$

Considering projections on the axis $O z$ of the indicated on the Fig. 3 forces, we have to take into account, that the shell is subjected to the bending deformation and as a result there arise small angles between direction of the forces $N_{x}$, as well as between directions of the forces $N_{y}$, acting on the opposite sides of the element. In consequence of this bending, projecting on the axis $O z$ of the normal forces $N_{x}$ implies

$$
-N_{x} d y \frac{\partial w}{\partial x}+\left(N_{x}+\frac{\partial N_{x}}{\partial x} d x\right)\left(\frac{\partial w}{\partial x}+\frac{\partial^{2} w}{\partial x^{2}} d x\right) d y
$$

where $w$ is the defection of the points of the middle plane. Neglecting the quantities of the smallness higher than the second order, we get

$$
\begin{equation*}
N_{x} \frac{\partial^{2} w}{\partial x^{2}} d x d y+\frac{\partial N_{x}}{\partial x} \frac{\partial w}{\partial x} d x d y \tag{1.6}
\end{equation*}
$$

Similarly, projecting on the axis $O z$ of the normal forces $N_{y}$ implies

$$
\begin{equation*}
N_{y} \frac{\partial^{2} w}{\partial y^{2}} d x d y+\frac{\partial N_{y}}{\partial y} \frac{\partial w}{\partial y} d x d y \tag{1.7}
\end{equation*}
$$

[^1]Concerning projections on the axis $O z$ of the forces $N_{x y}$ it should be mentioned that the slope of the bent middle surface in the direction $y$ on the opposite sides of the element will be expressed by $\frac{\partial w}{\partial y}$ on the one side and by $\frac{\partial w}{\partial y}+\frac{\partial^{2} w}{\partial x \partial y} d x$ on the opposite side, respectively. Hence, the projection on the axis $O z$ of the forces $N_{x y}$ will be

$$
\begin{equation*}
N_{x y} \frac{\partial^{2} w}{\partial x \partial y} d x d y+\frac{\partial N_{x y}}{\partial x} \frac{\partial w}{\partial y} d x d y \tag{1.8}
\end{equation*}
$$

Similarly, for the projection on the axis $O z$ of the forces $N_{y x}=N_{x y}$, we have

$$
\begin{equation*}
N_{y x} \frac{\partial^{2} w}{\partial x \partial y} d x d y+\frac{\partial N_{y x}}{\partial y} \frac{\partial w}{\partial x} d x d y \tag{1.9}
\end{equation*}
$$

Thus, in order to get the sum $q_{z}$ of all the projections on the axis $O z$ of the forces acting on the element under consideration we have to add together (1.6)(1.9) and the prescribed loading $q d x d y$. Taking into account (1.2), (1.3), we get

$$
\begin{equation*}
q_{z} d x d y:=\left(q+N_{x} \frac{\partial^{2} w}{\partial x^{2}}+N_{y} \frac{\partial^{2} w}{\partial y^{2}}+2 N_{x y} \frac{\partial^{2} w}{\partial x \partial y}-q_{x} \frac{\partial w}{\partial x}-q_{y} \frac{\partial w}{\partial y}\right) d x d y \tag{1.10}
\end{equation*}
$$

Now, substituting the expressions of $Q_{x}, Q_{y}$ from (1.4), (1.5) and of $q_{z}$ from (1.10) in (1.1), we arrive at

$$
\begin{aligned}
& \frac{\partial^{2} M_{x}}{\partial x^{2}}-2 \frac{\partial^{2} M_{x y}}{\partial x \partial y}+\frac{\partial^{2} M_{y}}{\partial y^{2}} \\
& =-\left(q+N_{x} \frac{\partial^{2} w}{\partial x^{2}}+N_{y} \frac{\partial^{2} w}{\partial y^{2}}+2 N_{x y} \frac{\partial^{2} w}{\partial x \partial y}-q_{x} \frac{\partial w}{\partial x}-q_{y} \frac{\partial w}{\partial y}\right) .
\end{aligned}
$$

Therefore, by virtue of

$$
\begin{align*}
& M_{x}=-D\left(\frac{\partial^{2} w}{\partial x^{2}}+\nu \frac{\partial^{2} w}{\partial y^{2}}\right), \quad M_{y}=-D\left(\frac{\partial^{2} w}{\partial y^{2}}+\nu \frac{\partial^{2} w}{\partial x^{2}}\right), \\
& M_{x y}=-M_{y x}=D(1-\nu) \frac{\partial^{2} w}{\partial x \partial y}, \\
& Q_{x}=\frac{\partial M_{y x}}{\partial y}+\frac{\partial M_{x}}{\partial x}, \quad Q_{y}=\frac{\partial M_{y}}{\partial y}-\frac{\partial M_{x y}}{\partial x},  \tag{1.11}\\
& Q_{x}^{*}:=Q_{x}+\frac{\partial M_{y x}}{\partial y}=\frac{\partial M_{x}}{\partial x}+2 \frac{\partial M_{y x}}{\partial y}, \\
& Q_{y}^{*}:=Q_{y}-\frac{\partial M_{x y}}{\partial x}=\frac{\partial M_{y}}{\partial y}-2 \frac{\partial M_{x y}}{\partial x}
\end{align*}
$$

$\left(D:=\frac{2 E h^{3}}{3\left(1-\nu^{2}\right)}\right.$ is the flexural rigidity, $\nu$ is Poisson's ratio, $E$ is Young's modulus), we obtain

$$
\begin{align*}
& \frac{\partial^{2}}{\partial x^{2}}\left[D\left(\frac{\partial^{2} w}{\partial x^{2}}+\nu \frac{\partial^{2} w}{\partial y^{2}}\right)\right]+2 \frac{\partial^{2}}{\partial x \partial y}\left[D(1-\nu) \frac{\partial^{2} w}{\partial x \partial y}\right] \\
& +\frac{\partial^{2}}{\partial y^{2}}\left[D\left(\frac{\partial^{2} w}{\partial y^{2}}+\nu \frac{\partial^{2} w}{\partial x^{2}}\right)\right]  \tag{1.12}\\
& =q+N_{x} \frac{\partial^{2} w}{\partial x^{2}}+N_{y} \frac{\partial^{2} w}{\partial y^{2}}+2 N_{x y} \frac{\partial^{2} w}{\partial x \partial y}-q_{x} \frac{\partial w}{\partial x}-q_{y} \frac{\partial w}{\partial y} .
\end{align*}
$$

For the deformation components of the middle plane we have (see, Timoshenko, Woinowsky-Krieger [1], §92)

$$
\begin{gather*}
\varepsilon_{x}=\frac{\partial u}{\partial x}+\frac{1}{2}\left(\frac{\partial w}{\partial x}\right)^{2}, \varepsilon_{y}=\frac{\partial v}{\partial y}+\frac{1}{2}\left(\frac{\partial w}{\partial y}\right)^{2}  \tag{1.13}\\
\varepsilon_{x y}=\frac{1}{2}\left(\frac{\partial u}{\partial y}+\frac{\partial v}{\partial x}+\frac{\partial w}{\partial x} \frac{\partial w}{\partial y}\right)
\end{gather*}
$$

where $u$ and $v$ are displacement vector components with respect to the axes $O x$ and $O y$ in the middle plane. Evidently, in view of (1.13),

$$
\begin{equation*}
\frac{\partial^{2} \varepsilon_{x}}{\partial y^{2}}+\frac{\partial^{2} \varepsilon_{y}}{\partial x^{2}}-2 \frac{\partial^{2} \varepsilon_{x y}}{\partial x \partial y}=\left(\frac{\partial^{2} w}{\partial x \partial y}\right)^{2}-\frac{\partial^{2} w}{\partial x^{2}} \frac{\partial^{2} w}{\partial y^{2}} \tag{1.14}
\end{equation*}
$$

After integration within the limits $-h(x, y)$ and $h(x, y)$ from Hooke's law we get the following expressions for the components of the deformation in the middle plane

$$
\begin{equation*}
\varepsilon_{x}=\frac{1}{2 h E}\left(N_{x}-\nu N_{y}\right), \quad \varepsilon_{y}=\frac{1}{2 h E}\left(N_{y}-\nu N_{x}\right), \quad \varepsilon_{x y}=\frac{1+\nu}{2 h E} N_{x y} \tag{1.15}
\end{equation*}
$$

since full deformations in the layer of the prismatic shell parallel to the middle surface on the distance $z$ can be written as follows (see Donnell [3], p.214):

$$
\begin{gathered}
e_{x}(x, y, z)=\varepsilon_{x}(x, y)-z \frac{\partial^{2} w(x, y)}{\partial x^{2}}, \\
e_{y}(x, y, z)=\varepsilon_{y}(x, y)-z \frac{\partial^{2} w(x, y)}{\partial y^{2}}, \\
e_{x y}(x, y, z)=\varepsilon_{x y}(x, y)-z \frac{\partial^{2} w(x, y)}{\partial x \partial y}, \\
e_{x z}(x, y, z)=0, e_{y z}(x, y, z)=0, e_{z}(x, y, z)=0, \\
N_{x}:=\int_{-h(x, y)}^{h(x, y)} \sigma_{x}(x, y, z) d z, N_{y}:=\int_{-h(x, y)}^{h(x, y)} \sigma_{y}(x, y, z) d z, N_{x y}:=\int_{-h(x, y)}^{h} \tau_{x y}(x, y, z) d z,
\end{gathered}
$$

where $\sigma_{x}, \sigma_{y}, \tau_{x y}$ are the stress tensor components.
Let us assume $q_{x}=0, q_{y}=0$ and introduce the stress function $F(x, y)^{3)}$ :

$$
\begin{equation*}
N_{x}=\frac{\partial^{2} F}{\partial y^{2}}, \quad N_{y}=\frac{\partial^{2} F}{\partial x^{2}}, \quad N_{x y}=-\frac{\partial^{2} F}{\partial x \partial y} . \tag{1.16}
\end{equation*}
$$

Substituting the expressions (1.16) in (1.15), we have

$$
\begin{align*}
& \varepsilon_{x}=\frac{1}{2 h E}\left(\frac{\partial^{2} F}{\partial y^{2}}-\nu \frac{\partial^{2} F}{\partial x^{2}}\right) \\
& \varepsilon_{y}=\frac{1}{2 h E}\left(\frac{\partial^{2} F}{\partial x^{2}}-\nu \frac{\partial^{2} F}{\partial y^{2}}\right)  \tag{1.17}\\
& \varepsilon_{x y}=-\frac{1+\nu}{2 h E} \frac{\partial^{2} F}{\partial x \partial y}
\end{align*}
$$

Obviously, the expressions (1.16) satisfy (1.2), (1.3) with $q_{x}=0, q_{y}=0$. Substituting (1.17) in (1.14), we arrive at

$$
\begin{align*}
& \frac{\partial^{2}}{\partial y^{2}}\left[\frac{1}{2 h E}\left(\frac{\partial^{2} F}{\partial y^{2}}-\nu \frac{\partial^{2} F}{\partial x^{2}}\right)\right]+\frac{\partial^{2}}{\partial x^{2}}\left[\frac{1}{2 h E}\left(\frac{\partial^{2} F}{\partial x^{2}}-\nu \frac{\partial^{2} F}{\partial y^{2}}\right)\right] \\
& +\frac{\partial^{2}}{\partial x \partial y}\left(\frac{1+\nu}{h E} \frac{\partial^{2} F}{\partial x \partial y}\right)=\left(\frac{\partial^{2} w}{\partial x \partial y}\right)^{2}-\frac{\partial^{2} w}{\partial x^{2}} \frac{\partial^{2} w}{\partial y^{2}} \tag{1.18}
\end{align*}
$$

Along with (1.12) equation (1.18) constitutes the system for determination of two unknown functions $F$ and $w$. In case $2 h(x, y)>0$, i.e., when the prismatic shell under consideration is non-cusped one, boundary conditions, e.g., for the edge normal to the axis $O x$, look like (see Donnell [3], p. 289, and also Awrejcewicz et al. [4]):

- for bending boundary conditions
either

$$
\begin{equation*}
w=0, \quad \frac{\partial w}{\partial x}=0 \tag{1.19}
\end{equation*}
$$

or

$$
\begin{equation*}
w=0, \quad M_{x}=0, \text { i.e., } D\left(\frac{\partial^{2} w}{\partial x^{2}}+\nu \frac{\partial^{2} w}{\partial y^{2}}\right)=0 \tag{1.20}
\end{equation*}
$$

or

$$
\begin{align*}
& M_{x}=0, \quad Q_{x}^{*}=0, \text { i.e., } D\left(\frac{\partial^{2} w}{\partial x^{2}}+\nu \frac{\partial^{2} w}{\partial y^{2}}\right)=0, \\
& 2 \frac{\partial}{\partial y}\left[D(1-\nu) \frac{\partial^{2} w}{\partial x \partial y}\right]+\frac{\partial}{\partial x}\left[D\left(\frac{\partial^{2} w}{\partial x^{2}}+\nu \frac{\partial^{2} w}{\partial y^{2}}\right)\right]=0 \tag{1.21}
\end{align*}
$$

[^2]- for membrane (tension-compression) boundary conditions either $u=0, v=0$ (when displacements in the middle plane are missing) or $N_{x}=0, N_{x y}=0$, i.e., $\frac{\partial^{2} F}{\partial y^{2}}=0, \frac{\partial^{2} F}{\partial x \partial y}=0$ (when resistance to the displacements in the middle plane is missing).

All the above homogeneous boundary conditions can be replaced by nonhomogeneous ones.

In the case of cusped prismatic shells their arise some peculiarities, which will be discussed in the next section.

## 2. Cylindrical Bending

If we consider the particular case of bending along the cylindrical surface with the axis parallel to the axis $O y, w$ will be dependent only on $x$, and for $q_{x}=0, q_{y}=0$ the quantities $\frac{\partial^{2} F}{\partial x^{2}}$ and $\frac{\partial^{2} F}{\partial y^{2}}$ will be constants. Evidently, equation (1.18) will be satisfied identically, while equation (1.12) will be simplified as follows:

$$
\begin{equation*}
\frac{\partial^{2}}{\partial x^{2}}\left(D \frac{\partial^{2} w}{\partial x^{2}}\right)=q+N_{x} \frac{\partial^{2} w}{\partial x^{2}} \tag{2.1}
\end{equation*}
$$

Indeed, from (1.2), (1,3) with $q_{x}=0, q_{y}=0$ it follows

$$
\frac{\partial N_{x}}{\partial x}=0, \quad \frac{\partial N_{x y}}{\partial x}=0 .
$$

Whence,

$$
\begin{equation*}
N_{x}=\text { const }, \quad N_{x y}=\text { const } . \tag{2.2}
\end{equation*}
$$

On the other hand, from (1.13) we have

$$
\varepsilon_{y}=0 .
$$

Therefore, by virtue of (1.15),

$$
\begin{equation*}
N_{y}=\nu N_{x}=\text { const. } \tag{2.3}
\end{equation*}
$$

Multiplying by $D$ both the sides of (2.1) and taking into account (1.11), we get

$$
\begin{equation*}
D \frac{\partial^{2} M_{x}}{\partial x^{2}}-N_{x} M_{x}=-q D \tag{2.4}
\end{equation*}
$$

After differentiation of (2.4), in view of (1.11), we obtain

$$
\begin{equation*}
\frac{\partial}{\partial x}\left(D \frac{\partial Q_{x}}{\partial x}\right)-N_{x} Q_{x}=-\frac{\partial q D}{\partial x} \tag{2.5}
\end{equation*}
$$

In the case of cusped prismatic shells, evidently, equations (1.12), (1.18), (2.4), (2.5) are singular differential equations and, therefore, setting of boundary conditions is characterized with some peculiarities (in some cases Dirichlet problem should be replaced by Keldysh problem (see Keldysh [5]) or by
weighted Dirichlet problem; Neumann problem should be replaced by weighted Neumann problem (see Bitsadze [6]). Evidently, boundary conditions (1.20), (1.21) will become weighted boundary conditions since

$$
D(0, y)=0 \text { and } D(x, y)>0 \text { for } x>0 .
$$

The classical bending problem within the framework of geometrically linear theory is investigated in detail in Jaiani [7]. His main results states that the boundary condition (1.19) can be set if and only if

$$
\int_{P}^{Q} D^{-1}(x, y) d y<+\infty
$$

(1.20) can be set if and only if

$$
\int_{P}^{Q} x D^{-1}(x, y) d y<+\infty
$$

where $P$ runs all the points of the cusped prismatic shell edge, e.g.,with the normal parallel to the axis $O x$; Q belongs to the shell middle plane, while (1.21) can be set without any restrictions. As we see from (2.4) and (2.5), in the case of geometrically nonlinear theory the analogous restrictions arise for the case of boundary conditions (1.21) as well. This topic will be discussed in detail in the forthcoming paper.

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Fig. 1


Fig. 3


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[^1]:    ${ }^{2)}$ If we take into account volume forces $(X, Y, Z)$ as well, then $q_{x}, q_{y}, q$ should be replaced by $q_{x}+X, q_{y}+Y, q+Z$. By bending deformation $q_{x}$ and $q_{y}$ are small quantities.

[^2]:    ${ }^{3)}$ Note that it differs from the following usual notion $N_{x}=2 h \frac{\partial^{2} F}{\partial y^{2}}, N_{y}=2 h \frac{\partial^{2} F}{\partial x^{2}}, N_{x y}=$ $-2 h \frac{\partial^{2} F}{\partial x \partial y}$.

